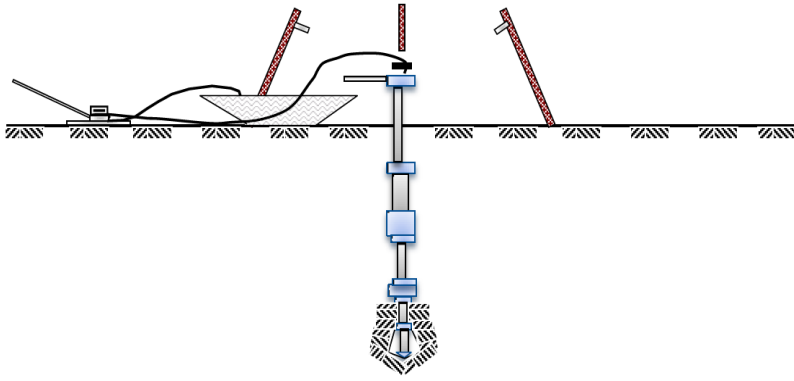


REPORT ON SOIL INVESTIGATION



:- NAME OF WORK :-

**CONSTRUCTION OF
PROPOSED
G +12 (TWELVE)
STORIED RESIDENTIAL
BUILDING**

LOCATION AT :-

**SEVOKE ROAD, OPPOSITE SIKKIM PLAZA, WARD NO. 40(SMC)
MOUZA DABGRAM, J.L.NO. 2 SHEET NO. R.S. 4 & 5, L.R. 4, R.S.
PLOT NO. 114/352,115/355,116,116/179 & L.R. PLOT NO.
-174,175,176,181 & 182**

R.S. KHATIAN

701/9,845/1,845/2,845/3,845/4,845/5,845/6,845/7,

845/8,845/9,845/10 L.R.KHATIAN NO.

1001,1002,1853,1854,1855(OLD)

1907 (NEW) ,P.S. BHAKTINGAR, DIST. JALPAIGURI.

CLIENTS:-

BUILWORTH DEVELOPERS

REPRESENTED BY ONE OF ITS PARTNER

**SRI MANISH KUMAR AGARWAL S/O. SRI DURGA PRASAD
AGARWAL.**

INVESTIGATOR



ACHARYA ASSOCIATES

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INTRODUCTION AND SCOPE :

Soil investigation has been carried out at SEVOKE ROAD , 3RD MILE for the purpose of designing suitable foundation for GROUND + 12 STORIED RESIDENTIAL BUILDING

The objective of the exploration work was to determine the probable sub surface conditions such as stratification, denseness or hardness of the strata, position of ground water table etc. and to evaluate probable range of safe bearing capacity for preparing safe and economic design of foundation.

The plot is more or less level and the Spot is same from existing road level. Two 150 mm dia bore holes were taken down to a depth of 8 m below the existing ground level at the site as per location shown in the site plan. Auger boring and bentonite mud drilling were used for drilling the holes. Standard Penetration Test was done on the soil at different depth.

Laboratory testing on selected undisturbed/representative soil samples were done for classification purpose and to determine their strength & other physical properties.

THE FOLLOWING TESTS WERE ONE FOR DETAILS SOIL INVESTIGATION :-

(A) FIELD TESTS:

1. Standard penetration tests.
2. Determination of In-Situ density.

(B) LABORATORY TESTS:

1. Natural moisture content
2. Specific Gravity
3. Grain size analysis.

1. Standard Penetration Tests :-

A standard split spoon sampler is driven 45 cm into the ground by means of a 63.5 kg hammer falling freely from a height of 75 cm. The total number of blows required to drive the second and third depth of 15 cm (i.e. total 30 cm) is called the standard Penetration resistance (N blows per 30 cm). After the blow counts are recorded, the spoon is withdrawn and a representative sample is obtained for identification tests. The N value has been corrected as per IS: 2123-1981.

Corrections:

- a) Due to Overburden – The N value for cohesion less soil shall be corrected for overburden (N').
- b) Due to Dilatancy – The value obtained after correction due to overburden shall be corrected for dilatancy if the stratum consists of fine sand and silt below water table for values of N' greater than 15, as under (N''):

$$N'' = 15 + \frac{1}{2} (N' - 15)$$

2. Determination of In-Situ density:

The in-situ density of soil is determined by core cutter method as per IS: 2720 (Part XXIX) - 1975.

(B) LABORATORY TESTS:

The soil samples collected from the bore holes during field Investigation were sent to the laboratory for determination of soil classification and physical properties.

The following laboratory tests were conducted on soil sample.

1. **Natural moisture content:** It is the ratio of weight of water in the voids to the weight of solids. It is expressed as percentage.

It is determined in the laboratory by Oven drying method as per IS: 2720 (Part-II)-1973. In this method the soil sample (collected in the air tight polythene pack) is dried in thermostatically controlled oven at 105-110°C for 24 hours.

2. **Specific Gravity:** Specific gravity is the ratio of the weight in air of a given volume of a material at a standard temperature to the weight in air of an equal volume of distilled water at the same stated temperature.

The specific gravity of soil sample is determined by density bottle method as per IS: 2720 (Part III/Sec 1) - 1980.

3. Shear Strength test:

When an external load is applied on a soil mass, shearing stresses are induced in it. If the shear stress developed on any plane in the soil exceeds a certain limiting value, failure of the soil occurs.

The maximum shear stress which a given soil can withstand is called its shear strength.

The factors governing the shear strength of a soil are :

(a) Internal friction.

(b) Cohesion.

As it is seen from two no's bore log data sheet that the average soil strata at 2 to 4 m is fine, medium & coarse sand, which is **cohesionless(C=0)**, so **shear parameter angle of internal friction (ϕ) is found out from correlation between angle of internal friction and corrected SPT value as per IS 6403 : 1981.**

Unconfined Compression test and Vane Shear test is applicable for pure forms of clay.

2. Grain size analysis :

The soil samples collected from the different depths were used for determination of Grain Size analysis. This is determined in the laboratory by the mechanical analysis, which consists of :-

(a) Dry mechanical analysis or sieve analysis.[IS-2720 (Part-4)- 1985]

(b) Wet mechanical analysis or hydrometer analysis. [IS-2720 (Part-4)- 1985]

Determination of Net Safe Bearing Capacity of Soil:

Net Safe Bearing capacity of soil is determined considering the following two aspects :

1. **Shear failure of soil as per IS:6403-1981:** Under this aspect calculations are made for both General Shear failure and Local Shear failure and appropriate value of the either, or a interpolated value as per void ratio is determined as the net safe bearing capacity from shear failure point of view.

2. **Allowable settlement as per IS: 8009 (Part-1)-1986:** Maximum permissible settlement for R.C.C. structure and the type of soil as mentioned in the report(sandy) is 50mm as per IS: 8009 (Part-1)-1986. In the present case considering all aspects, allowable settlement as indicated in the Net allowable bearing capacity Table has been assumed to determine the Net Safe bearing capacity by the formula suggested by Bowles (1988):

$$\text{Net Safe Bearing Capacity} = 48N_{\text{cor}}R_d((B+.33)/2)^2 S_aR_w$$

Where

N_{cor} = Design N (SPT) Value

S_a = Allowable Settlement

R_d = Depth Correction Factor =

B = Width of Footing

R_w = Water Table Correction

The **Net allowable bearing capacity** is taken as the lesser of the two values determined considering the above two aspects.

The calculations are shown in table- 3, 4, 5, 6 & 7

Table-1**BORE LOG DATA SHEET**

TYPE OF BORING		SHELL & AUGER		TYPE OF DRILLING		BORE HOLE NO. 1			
DIA OF BORE		150 MM		BMD		GROUND/ BED RL		The spot is same from road level.	
TERMINATION DEPTH		8.00 m				LOCATION			
COMMENCED ON : 30/07/2018		COMPLETED ON: 30/07/2018				3RD, Mile Sevoke Road, P.S.			
GROUND WATER LEVEL		0.91 M down from G.L				Bhaktinagar, dist. Jalpaiguri.			
DESCRIPTION OF STRATA		LEGEND	FROM	TO	Thickness	N	SAMPLES		DEPTH
			m	m	m	Value	Type	Ref. No.	m
Silty fine, medium, Course sand with Gravel grey in colour.		、	2.00	2.45	0.45	15	P	P-I/1	2.15-2.45
Do			3.00	3.45	0.45	27	P	P-I/2	3.15-3.45
Do			4.00	4.45	0.45	37	P	P-I/3	4.15-4.45
Silty fine, medium, Course sand with Gravel grey in colour.			5.00	5.45	0.45	57	P	P-I/4	5.15-5.45
Do			6.00	6.45	0.45	104	P	P-I/5	6.15-6.45
Do			7.00	7.45	0.45	122	P	P-I/6	7.15-7.45

Do		8.00	8.45	0.45	125	P	P-I/7	8.15-8.45
Code : U-Undisturbed sample, D – Disturbed Sample, L – Large Diameter, C – Core W-Water Sample, P-Penetration. Test, V – Vane Shear Test								
No. of disturbed Sample : NIL			No. of UDS : NIL			No. of Vane Test : NIL		
No. of Large Diameter Sample : NIL			No. of S.P.T. : SEVEN (07)			No. of Water Sample : NIL		

Table-2

BORE LOG DATA SHEET

TYPE OF BORING		SHELL & AUGER		TYPE OF DRILLING		BORE HOLE NO. 2			
DIA OF BORE		150 MM		BMD		GROUND/ BED RL		The spot is same from road level.	
TERMINATION DEPTH		8.00 m						LOCATION	
COMMENCED ON : 30/07/2018		COMPLETED ON : 30/07/2018						3 RD , Mile Sevoke Road, P.S.	
GROUND WATER LEVEL		0.91 m down from G.L						Bhaktinagar, dist. Jalpaiguri.	
DESCRIPTION OF STRATA		LEGEND	FROM	TO	Thickness	N	SAMPLES		DEPTH
			m	m	m	Value	Type	Ref. No.	M
Silty fine, medium, Course sand with Gravel grey in colour.		、	2.00	2.45	0.45	13	P	P-II/1	2.15-2.45
Do			3.00	3.45	0.45	27	P	P-II/2	3.15-3.45
Do			4.00	4.45	0.45	41	P	P-II/3	4.15-4.45

Silty fine, medium, Course sand with Gravel grey in colour.		5.00	5.45	0.45	57	P	P-II/4	5.15-5.45
Do		6.00	6.45	0.45	124	P	P-II/5	6.15-6.45
Do		7.00	7.45	0.45	120	P	P-II/6	7.15-7.45
Do		8.00	8.45	0.45	123	P	P-II/7	8.15-8.45
Code : U-Undisturbed sample, D – Disturbed Sample, L – Large Diameter, C – Core W-Water Sample, P-Penetration. Test, V – Vane Shear Test								
No. of disturbed Sample : NIL			No. of UDS : NIL			No. of Vane Test : NIL		
No. of Large Diameter Sample : NIL			No. of S.P.T. : SEVEN(07)			No. of Water Sample : NIL		

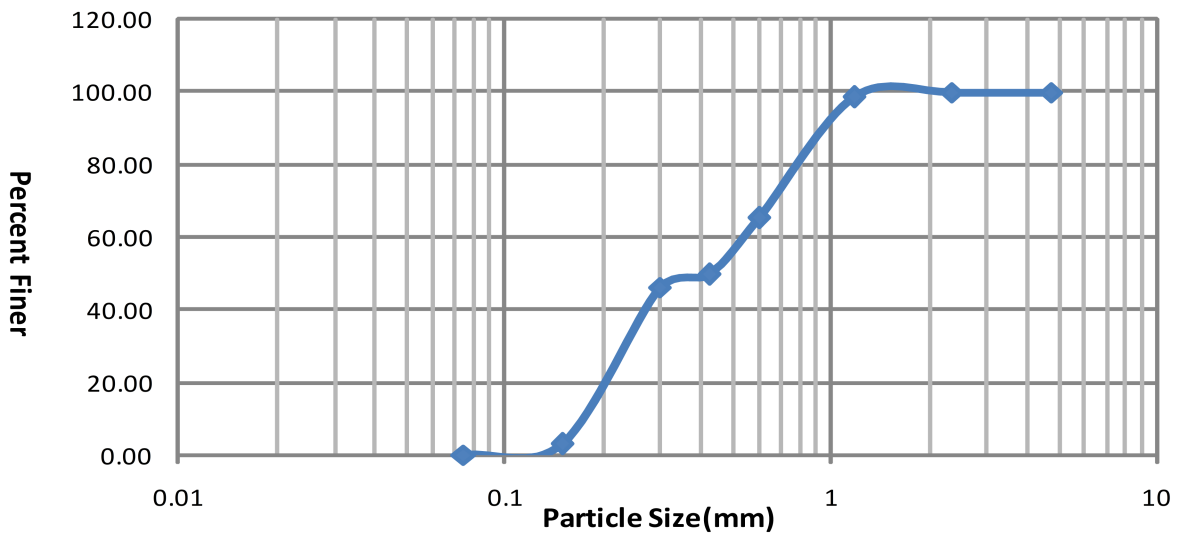
GRAIN SIZE ANALYSIS OF BORE HOLE 1 AT DEPTH 3 M

Total wt of sample =		329.2	gm			
Sieve size	Wt. of Sieve	Wt. of Sieve + soil	Wt. of soil	Percent retained	Cumulative retained	Percent finer
mm	gm	gm	gm	(%)	(%)	(%)
4.75	424.2	424.6	0.4	0.12	0.12	99.88
2.36	377.8	378.2	0.4	0.12	0.24	99.76
1.18	342	346	4	1.22	1.46	98.54
0.6	363.8	473.4	109.6	33.29	34.75	65.25
0.425	321.4	372.6	51.2	15.55	50.30	49.70
0.3	345.6	357.6	12	3.65	53.95	46.05
0.15	346.2	487.4	141.2	42.89	96.84	3.16
0.075	338	348.2	10.2	3.10	99.94	0.06

PAN

0.2

GRAIN SIZE ANALYSIS

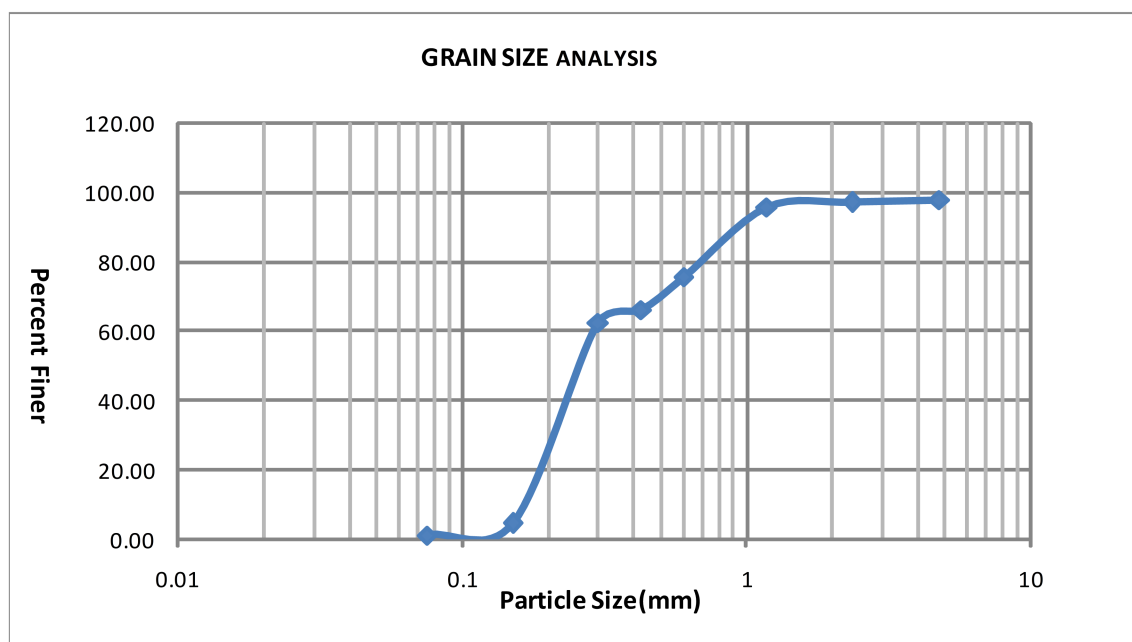


CLAY %	SILT %	SAND %			GRAVEL %
		FINE %	MEDIUM %	COARSE %	
0	0.06	49.64	50.06	0.12	0.12

100

$$\text{Uniformity Co-efficient (Cu)} = \frac{D_{60}}{D_{10}} = \frac{11}{3.11} = 3.54$$

GRAIN SIZE ANALYSIS OF BORE HOLE 2 AT DEPTH 3 M						
Total wt of sample		162.2 gm				
Sieve size	Wt. of Sieve	Wt.of Sieve + soil	Wt. of soil	Percent retained	Cumulati ve percent retained	Percent finer
mm	gm	gm	gm	(%)	(%)	(%)
4.75	424.2	427.4	3.2	1.97	1.97	98.03
2.36	377.8	379.2	1.4	0.86	2.84	97.16
1.18	342	344.4	2.4	1.48	4.32	95.68
0.6	363.8	396.4	32.6	20.10	24.41	75.59
0.425	321.4	336.6	15.2	9.37	33.79	66.21
0.3	345.6	351.8	6.2	3.82	37.61	62.39
0.15	346.2	439.8	93.6	57.71	95.31	4.69
0.075	338	343.4	5.4	3.33	98.64	1.36
PAN			2.2	1.36		



CLAY %	SILT %	SAND %			GRAVEL %
		FINE %	MEDIUM %	COARSE %	

Table-3

ESTIMATED PHYSICAL PROPERTIES OF SOIL		
Depth	2	3
Bulk Density, $\gamma_{\text{bulk}}(\text{t/m}^3)$	2.06	2.02
NatUral Moisture content, $w(\%)$	21.27	24.9
Natural dry density, $\gamma_{\text{dry}}(\text{t/m}^3)$	1.70	1.62
Specific Gravity, G	2.64	2.64
Void Ratio, e	0.55	0.63
Saturated density, $\gamma_{\text{sat}}(\text{t/m}^3)$	2.06	2.00
Submerged Density $\gamma_{\text{sub}}(\text{t/m}^3)$	1.06	1.00
Angle of Internal Friction(ϕ)	32.88	37.31
Angle of Internal Friction(ϕ')	23.41	27.05

Table-4

SPT(N) VALUE CORRECTION							
Depth(M)	Ybulk (gm /cc)	EOP	(C _n).	N	N'	N''	Design N Value
2	2.06	0.412	1.29	13	16.74	15.87	19.50
3	2.02	0.606	1.16	27	31.26	23.13	35.24
4	2.02	0.808	1.08	37	39.84	27.42	42.48
5	2.02	1.01	1.00	57	56.86	35.93	48.10
6	2.02	1.212	0.95	99	93.96	54.48	52.16
7	2.02	1.414	0.90	99	89.16	52.08	51.00
8	2.02	1.616	0.86	99	84.82	49.91	

N = Field N Value(Minimum of all bore holes)

N' = N Value after overburden correction

N'' = N Value after Dilatancy correction

EOP = Effective overburden pressure

C_n = Overburden correction

Table-5

Calculation of Net Safe Bearing Capacity as per IS-6403:1981, Based on General Shear Failure with ϕ determined from Design 'N' value, derived from corrected field N value as per IS- 2131:1981.

Width (B in metre)	2	3	4	3	4	6
Submerged Density of Soil(t/m ³)	1.06	1.06	1.06	1.00	1.00	1.00
Saturated Density of Soil(t/m ³)	2.06	2.06	2.06	2.00	2.00	2.00
Depth(D in metre)	2	2	2	3	3	3
Surcharge (q in t/ m ²)	2.11	2.11	2.11	3.01	3.01	3.01
ϕ (degree)	32.88	32.88	32.88	37.31	37.31	37.31
Water table correction(w^l)	0.5	0.5	0.5	0.5	0.5	0.5
Shape factors						
s_q	1.2	1.2	1.2	1.2	1.2	1.2
s_γ	0.8	0.8	0.8	0.8	0.8	0.8
Depth factors						
d_q	1.17	1.12	1.09	1.17	1.13	1.1
d_γ	1.17	1.12	1.09	1.17	1.13	1.1
Inclination factors						
i_q	1	1	1	1	1	1
i_γ	1	1	1	1	1	1
Bearing capacity factors						
N_q	26.97	26.97	26.97	47.58	47.58	47.58
N_γ	37.14	37.14	37.14	76.39	76.39	76.39
$q \cdot (N_q - 1) \cdot s_q \cdot d_q \cdot i_q$ (t/m ²)	76.95	73.66	71.68	197.10	190.36	185.31
$0.5 \cdot B \cdot \gamma_{sub} \cdot N_\gamma \cdot s_\gamma \cdot d_\gamma \cdot i_\gamma \cdot w^l$ (t/m ²)	35.72	51.29	66.56	107.50	138.43	202.1

Table-6

Calculation of Net Safe Bearing Capacity as per IS-6403:1981, Based on Local Shear Failure with ϕ' determined from Design 'N' value, derived from corrected field N value as per IS- 2131:1981.

Width (B in metre)	2	3	4	3	4	6
Submerged Density of Soil(t/m ³)	1.06	1.06	1.06	1.00	1.00	1.00
Saturated Density of Soil(t/m ³)	2.06	2.06	2.06	2.00	2.00	2.00
Depth(D in metre)	2	2	2	3	3	3
Surcharge (q in t/ m ²)	2.11	2.1105	2.11	3.01	3.01	3.014
ϕ' (degree)	23.41	23.41	23.41	27.05	27.05	27.05
Water table correction(w^I)	0.5	0.5	0.5	0.5	0.5	0.5
Shape factors						
s_q	1.2	1.2	1.2	1.2	1.2	1.2
s_γ	0.8	0.8	0.8	0.8	0.8	0.8
Depth factors						
d_q	1.17	1.12	1.09	1.17	1.13	1.1
d_γ	1.17	1.12	1.09	1.17	1.13	1.1
Inclination factors						
i_q	1	1	1	1	1	1
i_γ	1	1	1	1	1	1
Bearing capacity factors						
N'_q	9.31	9.31	9.31	13.83	13.83	13.83
N'_γ	9.14	9.14	9.14	15.60	15.60	15.60
$q.(N'_q - 1).s_q.d_q.i_q$ (t/m ²)	24.62	23.57	22.94	54.30	52.44	51.05
$0.5.B.\gamma_{sub}.N'_\gamma.s_\gamma.d_\gamma.i_\gamma.w^I$ (t/m ²)	8.79	12.62	16.38	21.95	28.27	41.28
Ultimate net bearing capacity(t/m ²)	33.41	36.19	39.31	76.25	80.71	92.33

Table-7

Summary Table of calculation of Net Safe Bearing Capacities based on Shear Failure and Settlement criteria as per Codal Provisions based on which Suggested Net Safe Bearing Capacity has been recommended.

Depth	Width(B) Metre	Length(L)	Net Safe Bearing Capacity Based on General Shear Failure(t/m^2)	Net Safe Bearing Capacity Based on Local Shear Failure(t/m^2)	Void Ratio	Net Safe Bearing Capacity Based on Void Ratio(t/m^2)	Net Safe Bearing Capacity Based on Allowable Settlement(25 to 50mm)(t/m^2)	Suggested Net Safe Bearing Capacity (t/m^2)
3 Mrtre(MAT)	6	6	101.53	25.42	0.63	70.19	36.57	36.57
	6	6	76.15	19.06	0.63	52.64	36.57	36.57
	6	6	109.60	26.90	0.63	75.55	36.57	36.57

Recommendations:-

- 1. Above recommendations are made for Raft type footings of mentioned minimum size(6mx 6m) and depth.*
- 2. Observation valid only for specified bore hole positions.*
- 3. Exact settlement calculation should be done for foundation design.*

PHOTOGRAPH



